

THEODOLITE SURVEYING

STEFFI STEPHEN

Asst. Professor, PHCET

INTRODUCTION

- Theodolite is an intricate instrument mainly used for accurate measurement of horizontal & vertical angles
- Because of its various uses it is sometimes known as "Universal Instrument"
- Different purposes
 - Horizontal angle
 - Vertical angle
 - Deflection angle
 - Magnetic bearing
 - Horizontal distance between points
 - Vertical height of an object
 - Difference of elevation between various points
 - Ranging of a line





CLASSIFICATION OF THEODOLITE

- Primarily classified as:
 - 1. Transit Theodolite
 - In transit theodolite, the telescope can be revolved through a complete revolution about its horizontal axis in a vertical plane
 - 2. Non-transit theodolite
 - Here, the telescope cannot be revolved through complete revolution in the vertical plane
- Theodolites can also be classified as
 - 1. Vernier theodolite
 - 2. Micrometer theodolite



PARTS & ITS FUNCTION



https://www.youtube.com/watch?v=CBlhQ76LAyl

BASIC DEFINITIONS

- 1. Centring
 - -Setting of a theodolite exactly over a station mark by means of a plumb bob
- 2. Transiting or Plunging of telescope

- Turning the telescope about its horizontal axis in a vertical plane through 180 degrees. Transiting results in change of face

3. Face Right/ Left

- Face left means the vertical circle of theodolite is on the left of the observer at the time of reading

4. Swinging the telescope

-turning the telescope in a horizontal plane. It is called right swing if the telescope is turned clockwise and left swing if telescope is turned anti clockwise

5. Line of collimation

- Imaginary line passing through optical centre of objective glass and its continuation

TEMPORARY ADJUSTMENTS

- 1. Setting up of theodolite over a station
- 2. Levelling Up
- 3. Focusing of eye piece & elimination of parallax
 - Focusing of eyepiece: clear and distinct view of cross hairs
 - Focusing of object glass : parallax elimination i.e., the image of object formed will be in the plane of cross hairs







Focus the crosshairs (using the Eyepiece) Focus the object (using the Focussing screw)

FUNDAMENTAL AXES OF THEODOLITE





The desired relationships between the fundamental lines are as follows:

- The axis of the plate level must be perpendicular to the vertical axis
- The line of collimation should coincide with the optical axis of the telescope and should also be perpendicular to the vertical axis.
- The axis of telescope must be parallel to the line of collimation.
- The line of collimation must be perpendicular to the horizontal axis. And the vertical circle should read zero when the line of collimation is horizontal.

MEASUREMENT OF ANGLES

- 1. To measure horizontal angle (Repetition Method)
- 2. To measure horizontal angle (Reitration Method)
- 3. To measure deflection angle
- 4. To measure vertical angle







5	Station	Object	Angle	Observation	Reading on	vernier	Angle on v	emier	Mean angle	Mean angle of observation	Remark
Differ					A	В	A	В	of vernier		
	Î	2	3	4	5	6	7	8	9	10	11
		A			0°0′0″	180°0′0″	200201208	201201 107	200201208		
ALC: N	0	В	ZAOB	Face left	30°20′20″	210°20′40″	30-20-20	30*20 40*	30~20 30"		1.0
	a de	A			0°0′0″	180°0′0″				30°20'30'	
0	0	B	ZAOB	Face right	30°20′40″	210°20'20"	30°20'40"	30°20′20″	0'20" 30°20'30"	· ·	

THEODOLITE TRAVERSE

INTRODUCTION

- Methods of traversing
 - Measurement of angles between successive lines
 - Included angles
 - Direct angles or angles to the right
 - Deflection angles
 - Direct measurement of bearings of survey line
 - Used for small works

TRAVERSE COMPUTATION

Theodolite traverse is plotted by computing latitude and departures

LATITUDE

- Latitude of a line is the distance measured parallel to the N-S line
- Latitude is positive when measured towards North (called as Northing) and negative towards south (southing)
- Latitude of a line = $1 \cos\theta$ where 1 is the length of the line and θ is the reduced bearing of the line

DEPARTURE

- Departure of a line is the distance measured parallel to E-W Line
- Departure is positive when measured towards East (called as Easting) and negative towards west (westing)
- Departure of a line = $1 \sin \theta$ where 1 is the length of the line and θ is the reduced bearing of the line



Line	Length (L)	Reduced Bearing (O)	Latitude (LCOS O)	Departure (L Sin O)
AB	L	NOE	+ L cos O	+ L sin O
BC	L	SOE	-L cos O	+ Lsin O
CD	L	SOW	-L cos O	-L sin O
DA	L	NOW	+ Lcos O	-L sin O

Line	Length	Reduced Bearing		Consect	itive Coordi	nates
	(L.)	(O)	Northing (+)	Southing (-)	Easting (+)	Westing (-)
AB	L	NOE	$L\cos\Theta$		$L\sin\Theta$	
BC	L	SOE		Lcos O	Lsin O	
CD	L	SOW		Lcos O		Lsin O
DA	L	NOW	$L\cos\!\Theta$			Lsin O

Check for Closed Traverse Sum of Northing- Sum of Southing Sum of Easting- Sum of Westing

CO-ORDINATES

- CONSECUTIVE CO-ORDINATES
 - Latitude and departure of any point with reference to the preceding point
- INDEPENDENT CO-ORDINATES
 - Co-ordinates of any point wrt a common origin
 - It may be obtained by adding algebraically the latitudes or departures of the lines between the points and the origin

BALANCING THE TRAVERSE

- In a closed traverse, the sum of northings must be equal to that of southings & sum of eastings should be equal to westings
- But in actual practice, some closing error always exist in traverse computation
- This is eliminated by balancing the traverse
- Here total errors in latitude and departure is computed and these errors are then distributed between traverse stations proportionately according to certain rules
- Rules for balancing a traverse
 - Bowditch's Rule
 - Transit Rule
 - Modified Transit Rule

BOWDITCH'S RULE

- Also called as compass rule
- Used to balance traverse when the linear and angular measurements are equally precise
- By this rule, the total error in latitude & departure is distributed in proportion to the lengths of the sides
- Correction in latitude/ departure

= Total error in latitude/ departure x <u>Length of that side</u> Perimeter of traverse

TRANSIT RULE

- Used to balance the traverse when angular measurements are more precise than linear measurements
- Correction in latitude

= Total error in latitude x <u>Latitude of that side</u> <u>Arithmetic sum of all latitudes</u>

• Correction in departure

= Total error in departure x *departure of that side Arithmetic sum of all departures*

Question 1

Calculate latitudes, departures & closing error for the following traverse and adjust using Bowditch's rule

Line	Length	WCB
AB	89.31	45°10′
BC	219.76	72°05′
CD	151.18	161°52′
DE	159.10	228°43′
EA	232.26	300°42′

Line	Length	WCB	RB Latitude Departure		Latitude		e
				Ν	S	Е	W
AB	89.31	45°10′	N45°10′E	62.967		63.335	
BC	219.76	72°05′	N72°05′E	67.605		209.10	
CD	151.18	161°52′	S18 8E		-143.67	47.05	
DE	159.10	228°43′	S48 43W		-104.97		-119.556
EA	232.26	300°42′	N 59 18W	118.578			-199.709
Perimeter – 851.61			Sum	249.15	248.64	319.485	319.265

Total error in latitude = Sum of northings – Sum of southings

= 249.15 - 248.64 = +0.51

Since error is positive correction will be negative

Total error in departure = Sum of E - Sum of W

= 319.485 - 319.265 = +0.22, Correction -ve

Correction in latitude/ departure

= Total error in latitude/ departure x <u>Length of that side</u> <u>Perimeter of traverse</u>

Correction in Latitude

Correction in line AB = $0.51 \text{ x} \frac{89.31}{851.61} = 0.0534$

Similarly in BC = 0.1316, CD = 0.0905, DE = 0.0952 & EA = 0.1390

Corrected latitude of line AB = 62.967 - 0.0534 = 62.9136

Similarly latitude of BC = 67.4734, CD = -143.7605, DE = -105.0652 & EA = 118.439

Correction in Departure

Correction in line AB = $0.22 \text{ x} \frac{89.31}{851.61} = 0.0230$

Similarly in BC = 0.0567, CD = 0.0390, DE = 0.0411 & EA = 0.060

Corrected departure of line AB = 63.335 - 0.0230 = 63.312

Similarly departure of BC = 209.0433, CD = 47.011, DE = -119.5971& EA = -199.769

Question 2

An abstract from a traverse sheet for a closed traverse is given below. Balance the traverse using transit rule

Line	Latitude	Departure
AB	-173.20	100
BC	-314.50	128.40
CD	86.60	50
DE	250.00	0
EA	154.90	-280

Line	Latitude	Departure
AB	-173.20	100
BC	-314.50	128.40
CD	86.60	50
DE	250.00	0
EA	154.90	-280
Sum	979.2	558.4

Error in latitude = -173.20-314.50+86.60+250+154.90 = +3.8 (correction -ve)

Error in departure = 100+128.4+50+0-280 = -1.6 (correction +ve)

Correction in latitude= Total error in latitude x <u>Arithmetic sum of all latitudes</u>

Correction in Latitude

Correction in line AB = $3.8 \times \frac{173.2}{979.2} = 0.6721$

Similarly in BC = 1.2204, CD = 0.3360, DE = 0.9701 & EA = 0.6011

Corrected latitude of line AB = -173.20-0.6721 = -173.8721

Similarly latitude of BC = -315.7204, CD = 86.264, DE = 249.0299& EA = 154.2989

• Correction in departure = Total error in departure x $\frac{departure of that side}{Arithmetic sum of all departures}$

Correction in Departure

Correction in line AB = $1.6 \text{ x} \frac{100}{558.4} = 0.2865$

Similarly in BC = 0.3679, CD = 0.1432, DE = 0 & EA = 0.8022

Corrected latitude of line AB = 100+0.2865 = 100.2865

Similarly latitude of BC = 128.7679, CD = 50.1432, DE = 0 & EA = -279.1978

Question 3

Balance the given traverse

Line	Length	Bearing
PQ	159.10	S 18°08' E
QR	232.26	S 48°43' W
RS	89.31	N 59°18' W
ST	219.76	N 45°10' E
TP	151.18	N 72°05' E

OMITTED MEASUREMENTS

Various Cases

- Lengths of two sides omitted
- Bearings of 2 sides omitted
- Length of one side and bearing of other side omitted
- Bearing of one side omitted
- Length of one side omitted
- Bearing & length of one side omitted

Q1) A traverse ABCDA was run but due to an obstruction between the stations A & B, it was not possible to measure the length and direction of line AB. The following data could only be obtained

Line	Length	RB
AD	44.5	N50°20'E
DC	67.0	S69°45′E
СВ	61.3	S30°10′E
BA	Ś	Ś

Step 1: Find L & D

Line	Length	RB	Latitude	Departure
AD	44.5	N50°20'E	28.405	34.25
DC	67.0	S69°45′E	23.18	62.85
СВ	61.3	S30°10′E	52.99	30.804
BA	Ś	Ś	L	D

Step 2:

For closed traverse, $\sum L = 0$ i.e., 28.40+23.18+52.99+L =0 L = -104.57 Also, $\sum D = 0$ 34.25+62.85+30.80+D = 0 D = -127.9 Step 3: Bearing of line AB

$$\tan\theta = \frac{D}{L} = \frac{127.9}{104.57} = 50^{\circ}43'51''$$

Bearing of line $AB = S 50^{\circ}43'51''W$

Step 4: Length of line AB

 $1 = \sqrt{L^2 + D^2} = \sqrt{-104.57^2 + -127.9^2} = 165.20 \text{ m}$

Check $L = 1 \cos\theta$

i.e., $104.57 = 1 \cos(50^{\circ}43'51'')$

l = 165.20 m

Q2) Due to some problems with equipment, the bearing of two sides were not taken for a closed traverse ABCDEA. From the available data, compute the missing data

Line	Length	RB
AB	230.5	S36°45′E
BC	250.2	S82°48′E
CD	210.8	S10°15′E
DE	240.3	Ś
EA	265.4	Ś

Step 1: Considering lines from AB to DE

Line	Length	RB	Latitude	Departure
AB	230.5	\$36°45′E	-184.68	137.91
BC	250.2	\$82°48'E	-31.358	248.22
CD	210.8	S10°15′E	-207.42	37.51
DE	240.3	Ś	L1	D1
alogad thereas	$\nabla I = 0$			

For closed traverse, $\Sigma L = 0$,

-423.45 + L1 = 0 L1 = 423.45

Also, $\sum D = 0$,

 $423.64 + D1 = 0 \qquad D1 = -423.64$

Bearing of line DE

 $\tan\theta = \frac{D}{L} = \frac{423.64}{423.45} = 45^{\circ}0'46''$

Bearing of line $AB = N 45^{\circ}0'46''W$

Step 2: Considering lines from AB to EA

Line	Length	RB	Latitude	Departure
AB	230.5	\$36°45'E	-184.68	137.91
BC	250.2	\$82°48′E	-31.358	248.22
CD	210.8	S10°15'E	-207.42	37.51
DE	240.3	N45°0′46‴W	169.874	-169.922
EA	265.4	Ś	L2	D2

For closed traverse, $\sum L = 0$,

-253.6+L2=0 L2=253.6

Also, $\sum D = 0$,

253.68 + D2 = 0 D2 = -253.68

Bearing of line EA

 $\tan\theta = \frac{D}{L} = \frac{253.68}{253.6} = 45^{\circ}0'30.5''$

Bearing of line $AB = N 45^{\circ}0'30.5''W$

Q3) The details of a part of theodolite survey are as under. Calculate the distance between a point P on AB 60 m from A and a point Q on CD 250 m from C and also determine the bearing of line PQ.

Line	Length	Bearing
AB	200	300°20′
BC	500	25°30′
CD	300	145°30′



Assume PBCQ as a closed traverse with co-ordinates of P as (0,0)

PB = 200-60 = 140 m CQ = 250 m

Line	Length	RB	Latitude	Departure
PB	140	N59°40′W	70.7	-120.83
BC	500	N25°30′E	451.29	215.26
CQ	250	S34°30′E	-206.03	141.60
QP	Ś	Ś	L	D

For closed traverse, $\Sigma L = 0$ i.e., 70.7+451.29-206.03+L=0 L = -315.96 mAlso, $\Sigma D = 0$ -120.83+215.26+141.6+D = 0 D = 236.03 mBearing of line PQ, $\tan \theta = \frac{D}{L} = \frac{236.03}{315.96} = \text{S} 36^{\circ}45'38'' \text{E}$

Length of line PQ, $1 = \sqrt{L^2 + D^2} = 394.39$ m



Q4) Find the length & bearing of line AB from the given co-ordinates

Point	Co-ordinates
А	975.50, 830.20
В	1189.70, 579.30

Latitude of AB = 1189.70-975.50 = 214.2

Departure of AB = 579.30-830.20 = -250.9

• Bearing of line AB, $\tan \theta = \frac{D}{L} = \frac{250.9}{214.2} = \text{N49°30'42''W}$

• Length of line AB, $1 = \sqrt{L^2 + D^2} = 329.897$ m

Gales traverse table

- Calculation for a closed traverse may be done in a tabular form known as gale's traverse table using following steps:
- 1. Sum up all included angles. There sum should be equal to $(2N\pm4)90^\circ$, where N is the number of sides. If there is error make necessary correction
- 2. From the given lengths & bearings, calculate consecutive co-ordinates
- 3. Take the sum of co-ordinates and see if correction is required. If yes, correct using balancing rules
- 4. From the corrected consecutive-co-ordinates calculate independent co-ordinates of points so that all are positive. Then the whole traverse will lie in the first quadrant.

	A	NGLI	E	N	cor A	REC	TED E				CON	CONSECUTIVE CO-ORDINATES				CORRECTION eLn OR eDn					CORRECTED CONSE, CO-ORDINATES				INDEPENDENT CO- ORDINATES	
INST				DIT				ш	臣	ai i	LAT= l,.	cos O _n	DEP=1	" sin O"	ш	LATI	TUDE	DEPA	RTURE	LATIT	TUDE	DEPA	RTURE	- 28		
STN.	0	0'	0"	CORREC	0	0'	0"	IN	LENG	W.C.	NORTHING	SOUTHING	EASTING	WESTING	INI	NORTHING	SOUTHING	EASTING	WESTING	NORTHING	SOUTHING	EASTING	WESTING	NORTHING	EASTING	
							1.5								8		S							2		
A	5	28	20	27'36	85	55	58																	5	5	
		1	(-1))				AB	BL 4	2	BL34	1	3.08		AB	0.05	ii ii	1.005		ML 29		3.075	-			
8	78	K.	40	25'23"	78	2	3														and the second			93.29	8.075	
i i		1.5	1.3				1 3	BC	3819	103	1	H.50	37.21		80	14	0.004	0.07	1		8.584	37.14		š	Q (d	
c	126	15	50	41'5"	126	56	55																	84.696	45.215	
6		1.9					. X	CD	83.8	157		77.14	32.74		CD	12	0.044	0.058			77.184	32,682		÷	8 - B	
D		42	55	22° 11°		5			States	-							1200000		and a state of the set		1.1017.1017		1000	7.512	77.807	
3		100 B						DA	72.8	265	<u>}</u>	2.51	8	72.78	DA		1.002	S	0.127		2.512)	72.887	2011 A		
2	_	5-8						- 3	() () () () () () () () () ()		ŝ	3 Q	8		\$i		2 - 2	\$ <u></u>	1	ž		8 8		5	5	
Σ Actual Angle	358	3	45	1° 56' 15"							∑ N⊨ 881.34	∑ S =881.2 4	∑E= 7 3.02	∑W=72.78		Σ ς ,=0.05	ΣC-=0.05	Σ 4 =0.133	ΣC=0.129	∑ N= 88.29	∑ S=88.29	∑E= 72.007	∑W=72.887		5	
											EL=∑N-	∑S=0.1	ED=SE-)	∑W=0.24	5 - P	Σ ς =Σς,	*Σc²=ατ	Σ € =Σ¢	+ΣC ₀ =0.28							

Q5) Calculate independent co-ordinates from the given consecutive co-ordinates

Line	Length	RB	Latit	ude	Departure				
			Ν	S	E	W			
AB	89.31	N45°10′E	62.967		63.335				
BC	219.76	N72°05′E	67.605		209.10				
CD	151.18	\$18° 8'E		-143.67	47.05				
DE	159.10	\$48°43'W		-104.97		-119.556			
EA	232.26	N59°18'W	118.578			-199.709			

Corrected	d Latitude	Corrected	Departure	Independent co-ordinates			
Ν	S	E	W	Ν	E		
62.9136		63.312		200	200		
67.4734		209.0433		267.4734	409.0433		
	-143.7605	47.011		123.7129	456.0543		
	-105.0652		-119.5971	18.6477	336.4572		
118.439			-199.769	137.0867	136.6882		

Q5) Calculate independent co-ordinates from the given consecutive co-ordinates

Line	Length	RB	Latit	ude	Departure				
			Ν	S	Е	W			
AB	89.31	N45°10′E	62.967		63.335				
BC	219.76	N72°05′E	67.605		209.10				
CD	151.18	S18° 8'E		-143.67	47.05				
DE	159.10	\$48°43'W		-104.97		-119.556			
EA	232.26	N59°18'W	118.578			-199.709			